STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	SF-040122	1	17

PROJECT: 17BP.11.R.131 REFERENCE: SF-040122

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY *ASHE*

PROJECT DESCRIPTION *BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER*

CONTENTS

PERSONNEL

SUMMIT

LANE, R.W.

INVESTIGATED BY *LANE, R.W.*

DRAWN BY *HILL, M.J.*

CHECKED BY *HUNSBERGER, W.S.*

SUBMITTED BY *FALCON ENG.*

DATE *AUGUST 2017*

CAUTION NOTICE

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2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS
(PAGE 1 OF 2)

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,

VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5, A-7-6			
SYMBOL														
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
MATERIAL PASSING #40 LL PI	— 6 MX		— NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	NO MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT PLASTIC RANGE (PI) PL — PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM — OPTIMUM MOISTURE SL — SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT, DPT, DMT, VST, PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS

S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [ ] CME-45C
- [ ] CME-55
- [ ] CME-550
- [ ] VANE SHEAR TEST
- [ ] PORTABLE HOIST
- [X] CME-450
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [ ] TUNG.-CARBIDE INSERTS
- [ ] CASING [ ] W/ ADVANCER
- [X] TRICONE 2 15/16" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [X] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC [ ] MANUAL

CORE SIZE:
- [ ] -B _____ [ ] -H _____
- [X] -N Q

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

PROJECT REFERENCE NO.	SHEET NO.
SF-040122	2A

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS
(PAGE 2 OF 2)

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)		NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)		FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)		FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)		COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIG,NAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

BORING ELEVATIONS COLLECTED USING "040122_ls_tnl.tin"

DATED 3/23/2016. ELEVATION: FEET

NOTES:

FIAD - FILLED IMMEDIATELY AFTER DRILLING

DATE: 8-15-14

PROJECT REFERENCE NO.	SHEET NO.
SF-040122	2B

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (PAGE 1 OF 2)

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)

From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.

STRUCTURE

SURFACE CONDITIONS

VERY GOOD Very rough, fresh unweathered surfaces

GOOD Rough, slightly weathered, iron stained surfaces

FAIR Smooth, moderately weathered and altered surfaces

POOR Slickensided, highly weathered surfaces with compact coatings or fillings or angular fragments

VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings

DECREASING SURFACE QUALITY ⇨

DECREASING INTERLOCKING OF ROCK PIECES ⇩

INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities

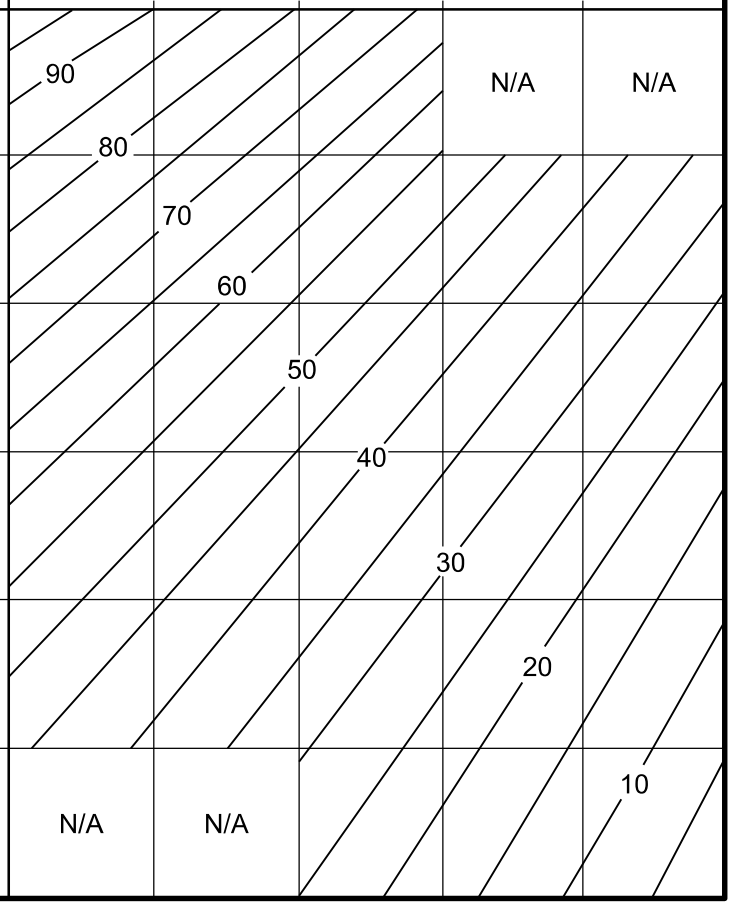
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets

VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets

BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity

DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces

LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes



PROJECT REFERENCE NO.	SHEET NO.
SF–040122	2C

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

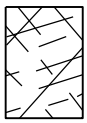
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (PAGE 2 OF 2)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)

From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.

COMPOSITION AND STRUCTURE

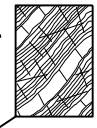


A. *Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.*

B. *Sand-stone with thin inter-layers of siltstone*

C. *Sand-stone and siltstone in similar amounts*

D. *Siltstone or silty shale with sand-stone layers*



E. *Weak siltstone or clayey shale with sandstone layers*

C, D, E, and **G** - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to **F** and **H.**

F. *Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure*

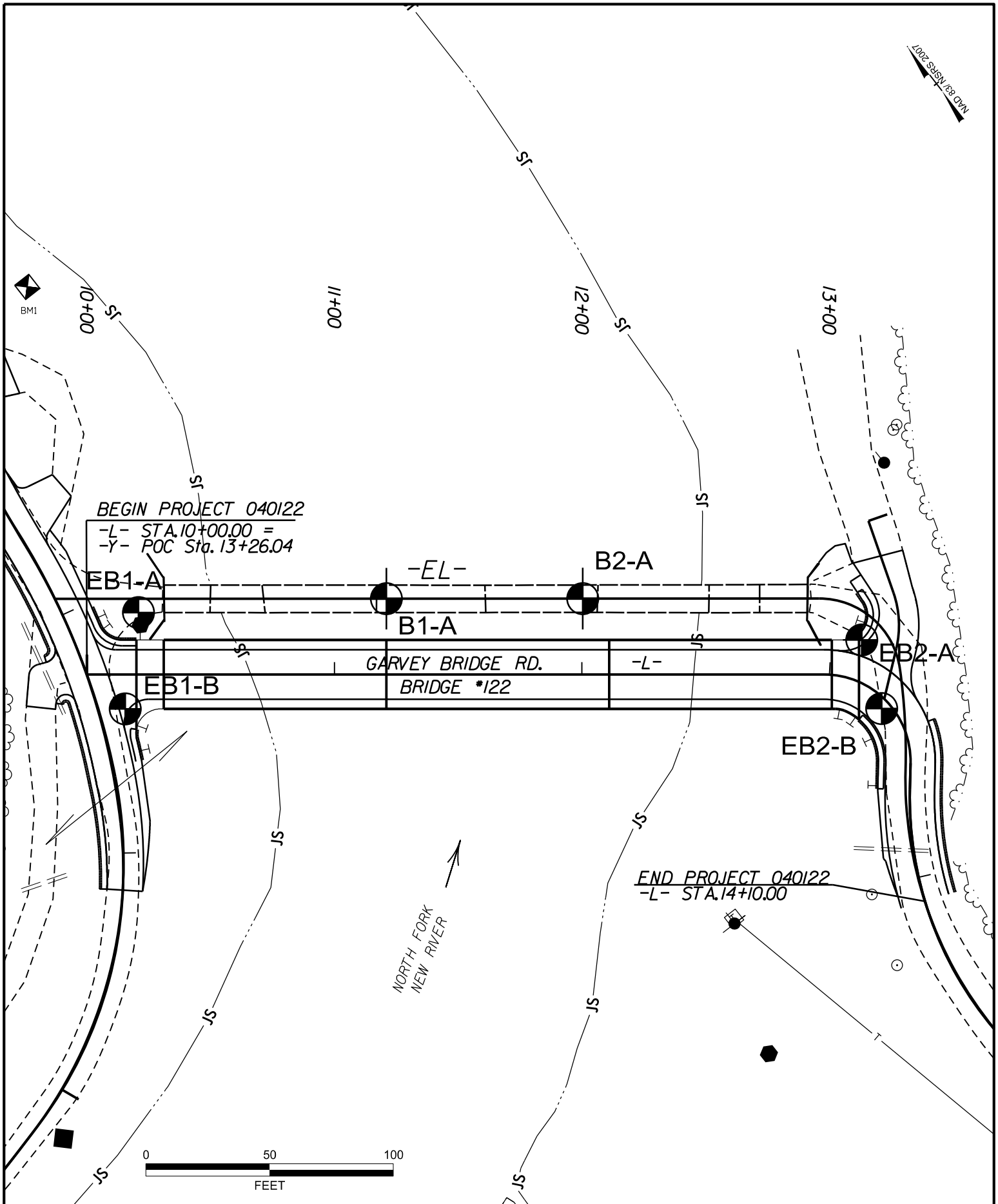
G. *Undisturbed silty or clayey shale with or without a few very thin sandstone layers*

H. ***Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.***

SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)	VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slicken-sided or highly weathered surfaces with soft clay coatings or fillings
COMPOSITION AND STRUCTURE					
Thick bedded, very blocky sandstone	70 / 60	A			
Sandstone with thin interlayers of siltstone; Sandstone and siltstone in similar amounts; Siltstone or silty shale with sandstone layers; Weak siltstone or clayey shale with sandstone layers		50 / 40	B	C	D / E
Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure			30	F	20
Undisturbed silty or clayey shale with or without a few very thin sandstone layers → Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.			G	H	10

→ Means deformation after tectonic disturbance

DATE: 8-19-16



NOTES:

- PLANS ADOPTED FROM ELECTRONIC SURVEY FILES RECEIVED FROM RK&K DATED MAY 2017.
- BRIDGE SKEW: 90°

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BORING LOCATION PLAN

BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.)
OVER NORTH FORK NEW RIVER
ASHE COUNTY, NORTH CAROLINA
WBS NO.: 17BP.11.R.131 | TIP NO.: SF-040122
FALCON PROJECT NO.: G16021.04

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.	
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER				**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 10+21	**OFFSET** 25 ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 2,490.0 ft	**TOTAL DEPTH** 21.8 ft	**NORTHING** 1,024,710	**EASTING** 1,308,899	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
DRILLER Moseley M.	**START DATE** 06/21/17	**COMP. DATE** 06/21/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
2490										ELEV. (ft) 2,490.0 DEPTH (ft) 0.0
	2,489.0	1.0	5	3	2	5		D		**ROADWAY EMBANKMENT** BROWN-TAN, SANDY SILT (A-4) WITH LITTLE GRAVEL
	2,486.5	3.5	5	7	4	11		D		
2485	2,484.0	6.0	3	3	3	6		D		
	2,481.5	8.5	6	4	4	8		M		
2480										2,478.0 12.0
	2,476.5	13.5	2	1	4	5		M		**ALLUVIAL** BROWN, SILTY SAND (A-2-4) MICACEOUS
2475										2,473.5 16.5
	2,471.5	18.5	100/0.5			100/0.5				2,472.5 BROWN, SAND AND GRAVEL (A-1-a) 17.5 **WEATHERED ROCK** BROWN AND GRAY, GNEISS
2470	2,468.2	21.8	60/0.0			60/0.0				2,468.2 21.8 Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,468.2 ft ON CR: GNEISS

GEOTECHNICAL BORING REPORT
BORE LOG

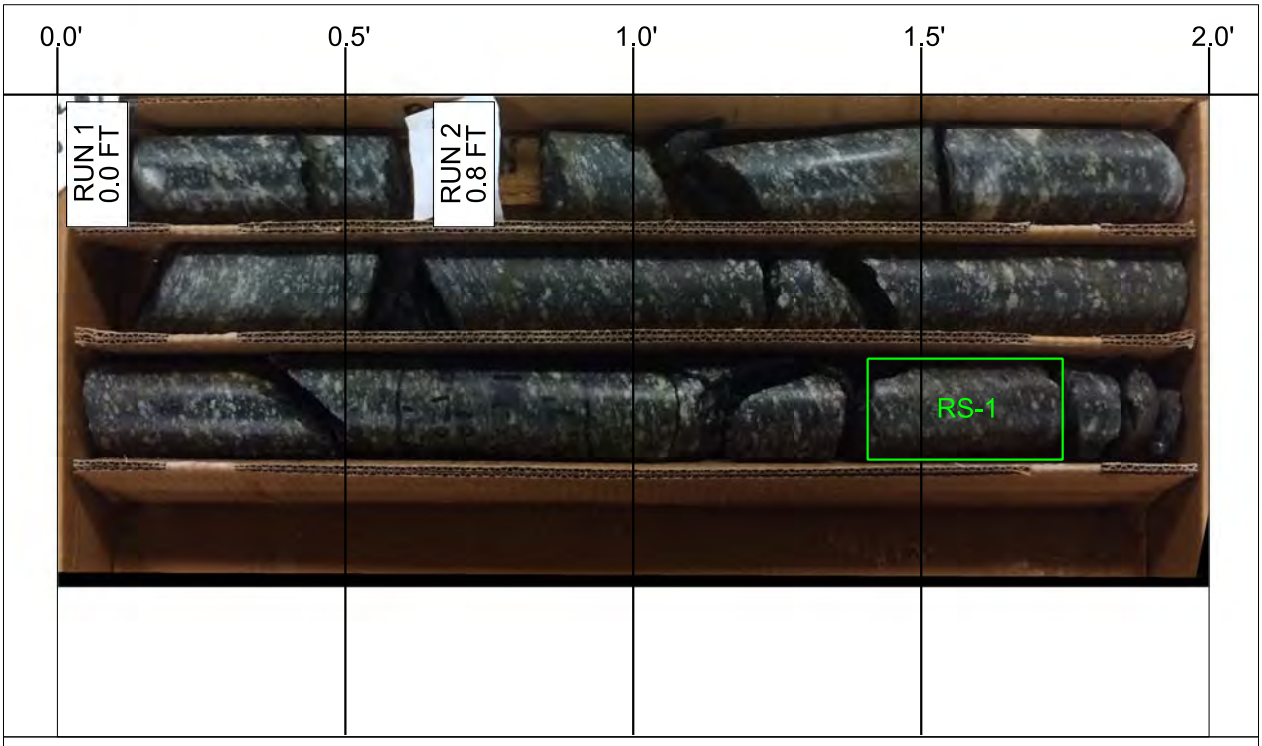
WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.	
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER				**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 10+15	**OFFSET** 14 ft RT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 2,489.5 ft	**TOTAL DEPTH** 20.0 ft	**NORTHING** 1,024,683	**EASTING** 1,308,871	**24 HR.** Dry
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
DRILLER Moseley M.	**START DATE** 06/20/17	**COMP. DATE** 06/20/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) DEPTH (ft)
2490										2,489.5 0.0
	2,488.5	1.0	3	3	2	5		D		**ROADWAY EMBANKMENT** BROWN SILT (A-4) WITH TRACE GRAVEL, MICACEOUS
	2,486.0	3.5						M		
2485			5	3	3	6				
	2,483.5	6.0	3	1	2	3		D		
	2,481.0	8.5								2,481.5 8.0
2480			3	2	3	5		D		**ALLUVIAL** BROWN SILT (A-4) WITH TRACE GRAVEL, MICACEOUS
	2,476.0	13.5								
2475			5	2	4	6		Sat.		
	2,472.0	17.5								2,472.0 17.5
	2,471.5	18.0								2,471.5 18.0 BROWN, SAND AND GRAVEL (A-1-a)
2470	2,469.5	20.0	100/0.3			100/0.3				**WEATHERED ROCK** GRAY, GNEISS 2,469.5 20.0
			60/0.0			60/0.0				Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,469.5 ft ON CR: GNEISS

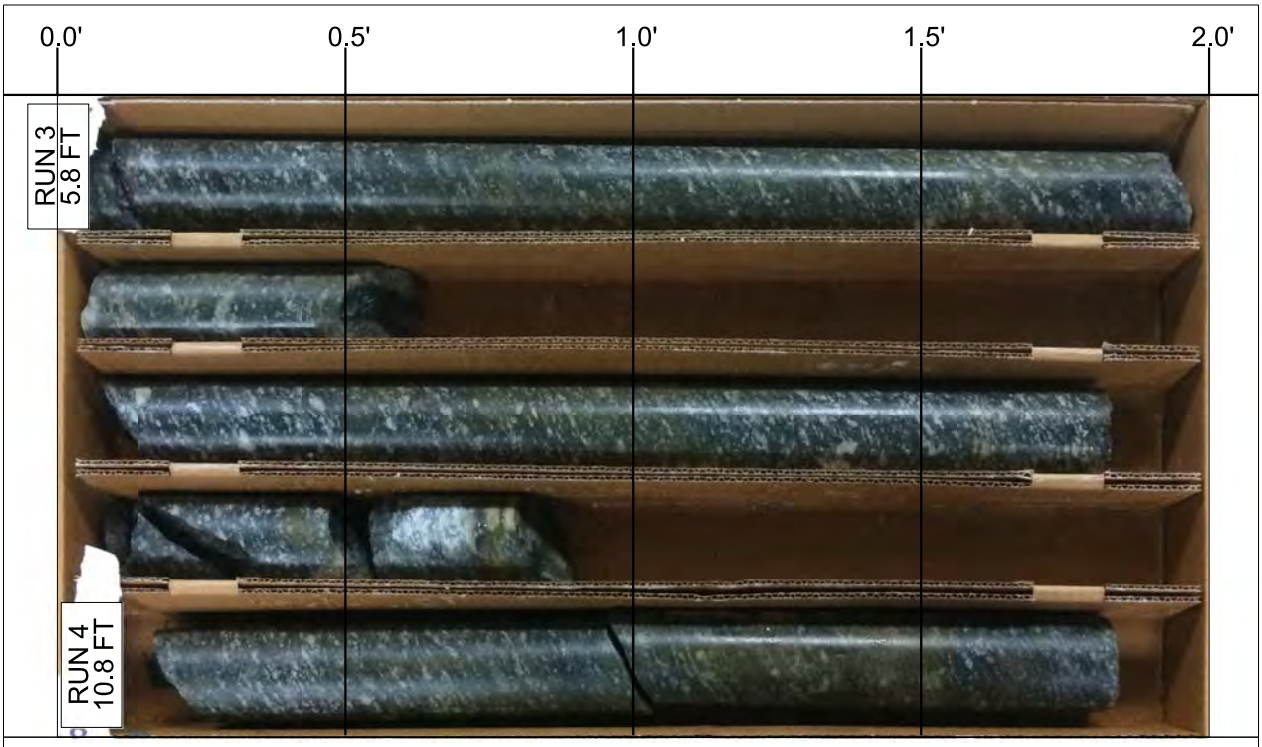
GEOTECHNICAL BORING REPORT
CORE LOG

WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER			**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 11+21	**OFFSET** 31 ft LT	**ALIGNMENT** -L-	**0 HR.** N/A
COLLAR ELEV. 2,469.9 ft	**TOTAL DEPTH** 12.5 ft	**NORTHING** 1,024,653	**EASTING** 1,308,982	**24 HR.** N/A
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Moseley M.	**START DATE** 06/20/17	**COMP. DATE** 06/20/17	**SURFACE WATER DEPTH** 2.7ft	
CORE SIZE NQ	**TOTAL RUN** 12.5 ft			

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
											ELEV. (ft) / DEPTH (ft)
2469.9											
	2,469.9	0.0	0.8	3:02/0.8	(0.5) 63%	(0.0) 0%		(5.5) 95%	(3.8) 66%		**CRYSTALLINE ROCK** FRESH, HARD, GRAY AND WHITE, GNEISS, CLOSE FRACTURE SPACING
	2,469.1	0.8	5.0	1:48/1.0 2:10/1.0 3:34/1.0 4:32/1.0 2:32/1.0	(5.0) 100%	(3.8) 76%					REC = 95%, RQD = 66%, GSI =75
2465	2,464.1	5.8									2,464.1 / 5.8
			5.0	4:11/1.0 3:48/1.0 3:58/1.0 3:46/1.0 3:38/1.0	(5.0) 100%	(4.7) 94%	RS-1	(6.7) 100%	(6.4) 96%		FRESH, HARD, GRAY AND WHITE, GNEISS, MODERATLEY CLOSE TO CLOSE FRACTURE SPACING
2460	2,459.1	10.8									REC = 100%, RQD = 96%, GSI =80
	2,457.4	12.5	1.7	5:55/1.0 2:26/0.7	(1.7) 100%	(1.7) 100%					2,457.4 / 12.5
											Boring Terminated at Elevation 2,457.4 ft IN CR: GNEISS



BORING B1-A, BOX 1 OF 2, 0.0 FEET TO 5.8 FEET.



BORING B1-A, BOX 2 OF 2, 5.8 FEET TO 12.5 FEET.

0 4" 8"
INCHES

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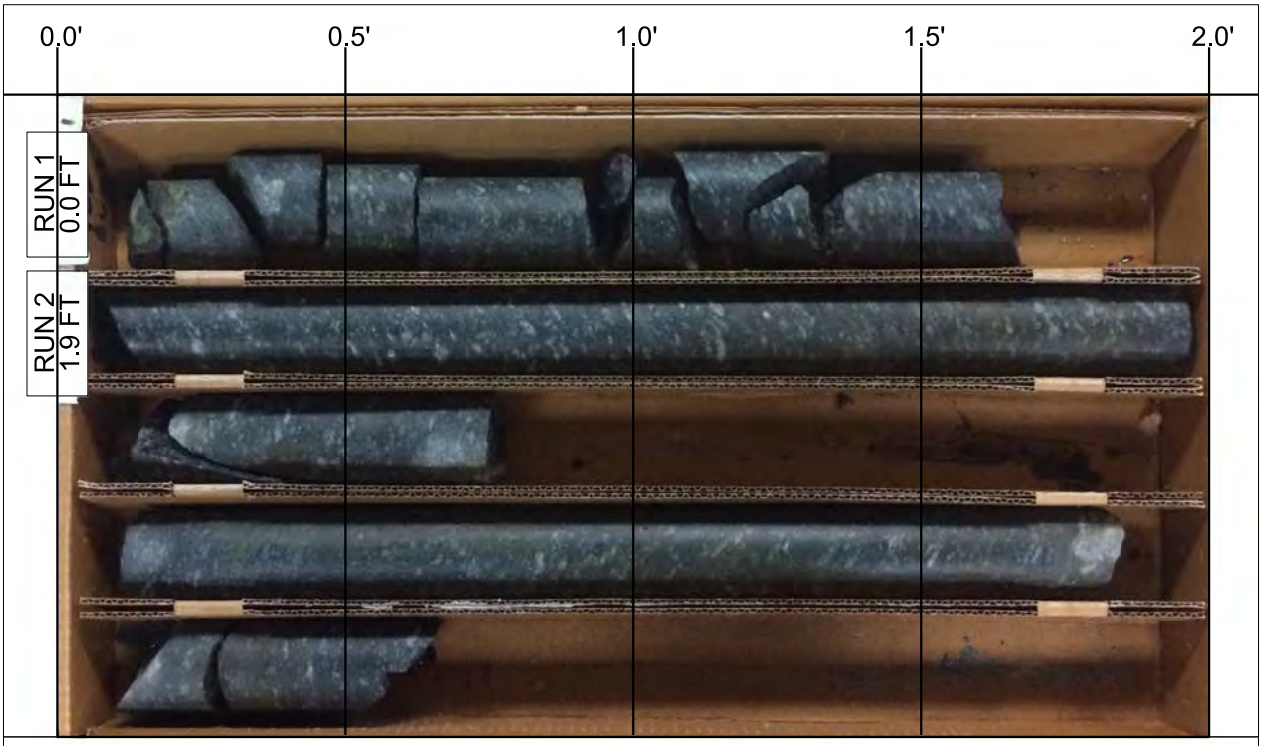
ROCK CORE PHOTOS

BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.)
OVER NORTH FORK NEW RIVER
ASHE COUNTY, NORTH CAROLINA
WBS NO.: 17BP.11.R.131 | TIP NO.: SF-040122
FALCON PROJECT NO.: G16021.04

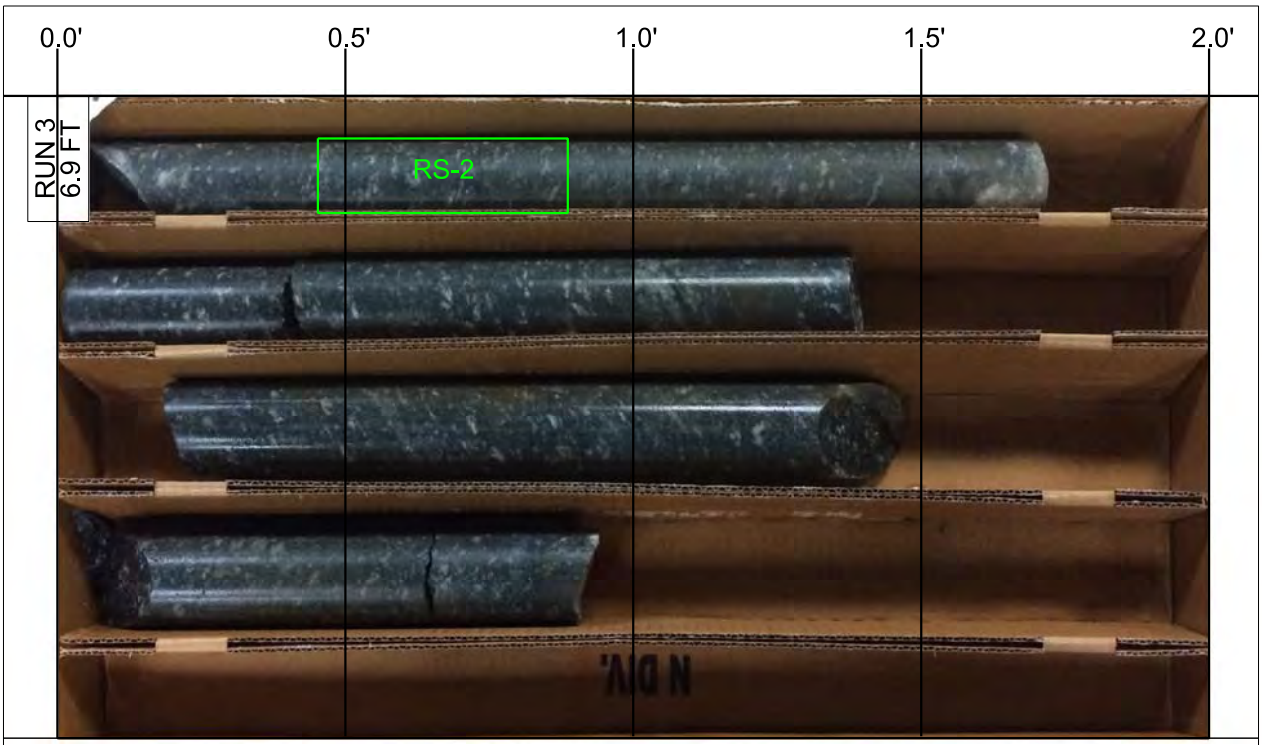
GEOTECHNICAL BORING REPORT
CORE LOG

WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.	
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER				**GROUND WTR (ft)**
BORING NO. B2-A	**STATION** 12+00	**OFFSET** 31 ft LT	**ALIGNMENT** -L-	**0 HR.** N/A
COLLAR ELEV. 2,471.2 ft	**TOTAL DEPTH** 16.9 ft	**NORTHING** 1,024,604	**EASTING** 1,309,044	**24 HR.** N/A
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
DRILLER Moseley M.	**START DATE** 06/21/17	**COMP. DATE** 06/21/17	**SURFACE WATER DEPTH** 1.3ft	
CORE SIZE NQ	**TOTAL RUN** 16.9 ft			

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
											ELEV. (ft) / DEPTH (ft)
2471.2											
2470	2,471.2	0.0	1.9	4:07/0.9	(1.6)	(0.0)		(1.6)	(0.0)		**CRYSTALLINE ROCK**
	2,469.3	1.9		4:15/1.0	84%	0%		84%	0%		2,469.3 FRESH, HARD, GRAY AND WHITE, GNEISS, VERY CLOSE TO CLOSE FRACTURE SPACING 1.9
			5.0	3:46/1.0	(5.0)	(4.1)		(15.0)	(13.8)		
				3:44/1.0	100%	82%		100%	92%		REC = 84%, RQD = 0%, GSI =55
				3:52/1.0							
2465				3:50/1.0							FRESH, HARD, GRAY AND WHITE, GNEISS, CLOSE TO MODERATLEY CLOSE FRACTURE SPACING
	2,464.3	6.9		3:50/1.0							
			5.0	3:41/1.0	(5.0)	(4.7)	RS-2				REC = 100%, RQD = 92%, GSI =80
				3:32/1.0	100%	94%					
				3:42/1.0							
2460				3:58/1.0							
	2,459.3	11.9		3:20/1.0							
			5.0	4:45/1.0	(5.0)	(5.0)					
				4:12/1.0	100%	100%					
				4:28/1.0							
2455				4:10/1.0							
	2,454.3	16.9		4:06/1.0							2,454.3 16.9
											Boring Terminated at Elevation 2,454.3 ft IN CR: GNEISS



BORING B2-A, BOX 1 OF 3, 0.0 FEET TO 6.9 FEET.



BORING B2-A, BOX 2 OF 3, 6.9 FEET TO 11.9 FEET.

0 4" 8"

INCHES

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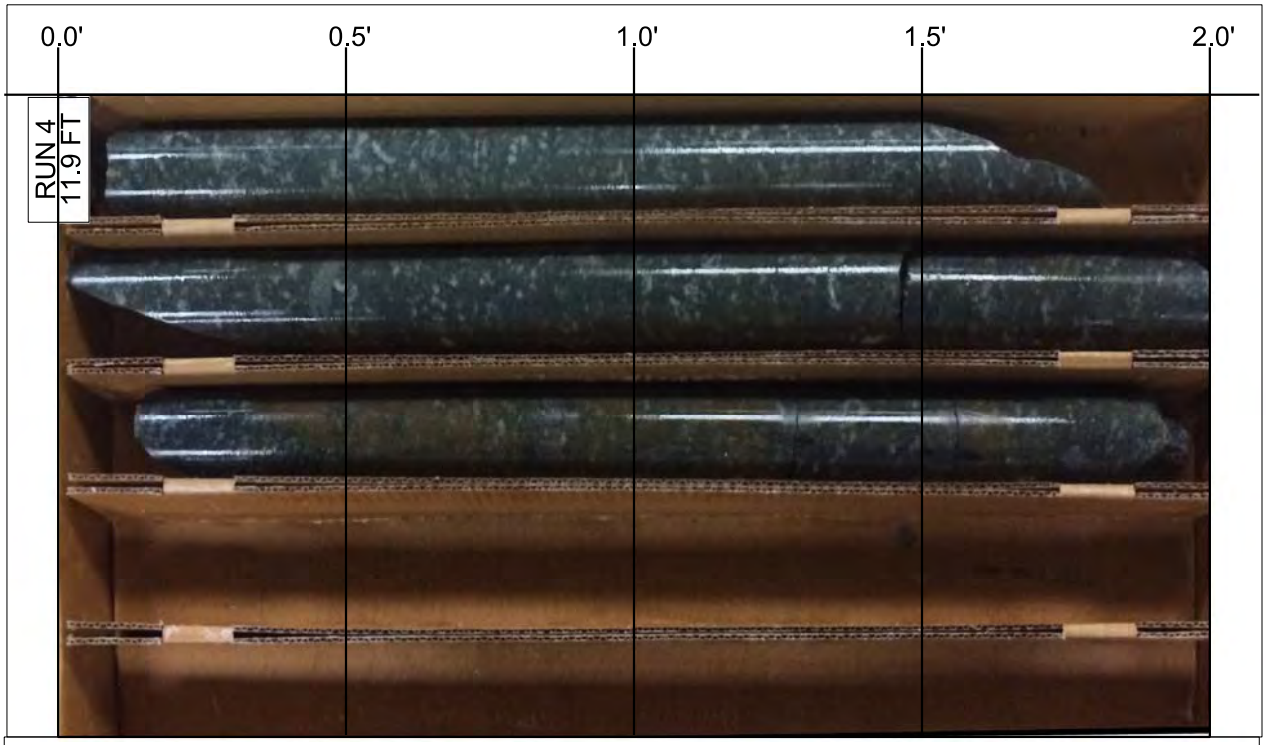
ROCK CORE PHOTOS

BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.)
OVER NORTH FORK NEW RIVER
ASHE COUNTY, NORTH CAROLINA
WBS NO.: 17BP.11.R.131 | TIP NO.: SF-040122
FALCON PROJECT NO.: G16021.04



BORING B2-A, BOX 3 OF 3, 11.9 FEET TO 16.9 FEET.

0 4" 8"

INCHES

FALCON ENGINEERING

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1210 TRINITY ROAD, SUITE 110
RALEIGH, NC 27607

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ROCK CORE PHOTOS

BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.)
OVER NORTH FORK NEW RIVER
ASHE COUNTY, NORTH CAROLINA
WBS NO.: 17BP.11.R.131 | TIP NO.: SF-040122
FALCON PROJECT NO.: G16021.04

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.	
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER				**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 13+08	**OFFSET** 16 ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 2,490.9 ft	**TOTAL DEPTH** 16.7 ft	**NORTHING** 1,024,521	**EASTING** 1,309,122	**24 HR.** 14.6
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic	
DRILLER Moseley M.	**START DATE** 06/19/17	**COMP. DATE** 06/19/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
2495										2,490.9	0.0
2490	2,489.9	1.0	4	4	3	7		M		**ROADWAY EMBANKMENT** BLACK, SANDY SILT (A-4) WITH TRACE GRAVEL AND LITTLE ORGANICS	
	2,487.4	3.5	2	2	3	5		M		2,487.9 BROWN, SILTY CLAY (A-6)	3.0
2485	2,484.9	6.0	3	2	2	4		W		2,484.9 **ALLUVIAL** BROWN, SILTY CLAY (A-6) MICACEOUS MOTTLED	6.0
	2,482.4	8.5	3	3	4	7		M			
2480										2,479.4	11.5
	2,477.4	13.5	5	6	11	17				2,478.4 GRAVEL (A-1-a) BROWN, SANDY SILT (A-4) WITH LITTLE GRAVEL	12.5
2475	2,474.2	16.7	60/0.0			60/0.0				2,474.9 2,474.2 **WEATHERED ROCK** GRAY, GNEISS	16.0 16.7
										Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,474.2 ft ON CR: GNEISS	

GEOTECHNICAL BORING REPORT
BORE LOG

WBS 17BP.11.R.131	**TIP** SF-040122	**COUNTY** ASHE	**GEOLOGIST** Lane, R. W.	
SITE DESCRIPTION BRIDGE NO. 122 ON SR 1549 (GARVEY BRIDGE RD.) OVER NORTH FORK NEW RIVER				**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 13+27	**OFFSET** 3 ft RT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 2,490.1 ft	**TOTAL DEPTH** 18.3 ft	**NORTHING** 1,024,494	**EASTING** 1,309,112	**24 HR.** Dry
DRILL RIG/HAMMER EFF./DATE SUM3359 CME-450 92% 11/05/2015		**DRILL METHOD** H.S. Augers		**HAMMER TYPE** Automatic
DRILLER Moseley M.	**START DATE** 06/19/17	**COMP. DATE** 06/19/17	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
2495										ELEV. (ft) 2,490.1 DEPTH (ft) 0.0
2490	2,489.1	1.0	5	5	4	9		M		**ROADWAY EMBANKMENT** DARK BROWN AND TAN, CLAYEY SANDY SILT (A-4) WITH TRACE ORGANICS, MICACEOUS
	2,486.6	3.5	2	1	2	3		M		
2485	2,484.1	6.0	3	2	4	6		M		2,484.1 6.0 **ALLUVIAL** TAN, SILTY CLAY (A-6) WITH TRACE ROCK FRAGMENTS, MICACEOUS
	2,481.6	8.5	4	5	5	10		M		
2480										2,478.1 12.0
	2,476.6	13.5	6	6	7	13		M		2,477.1 13.0 GRAVEL (A-1-a) TAN, SILTY CLAY (A-6) WITH TRACE ROCK FRAGMENTS, MICACEOUS
2475										2,474.1 16.0 **WEATHERED ROCK** GRAY, GNEISS
	2,471.8	18.3	60/0.0			60/0.0				2,471.8 18.3 Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at Elevation 2,471.8 ft ON CR: GNEISS

NCDOT BORE SINGLE G16021.04 BRIDGE 122.GPJ NC_DOT.GDT 8/25/17

FALCON ENGINEERING

UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE SPECIMENS

Performed in General Accordance with ASTM D7012

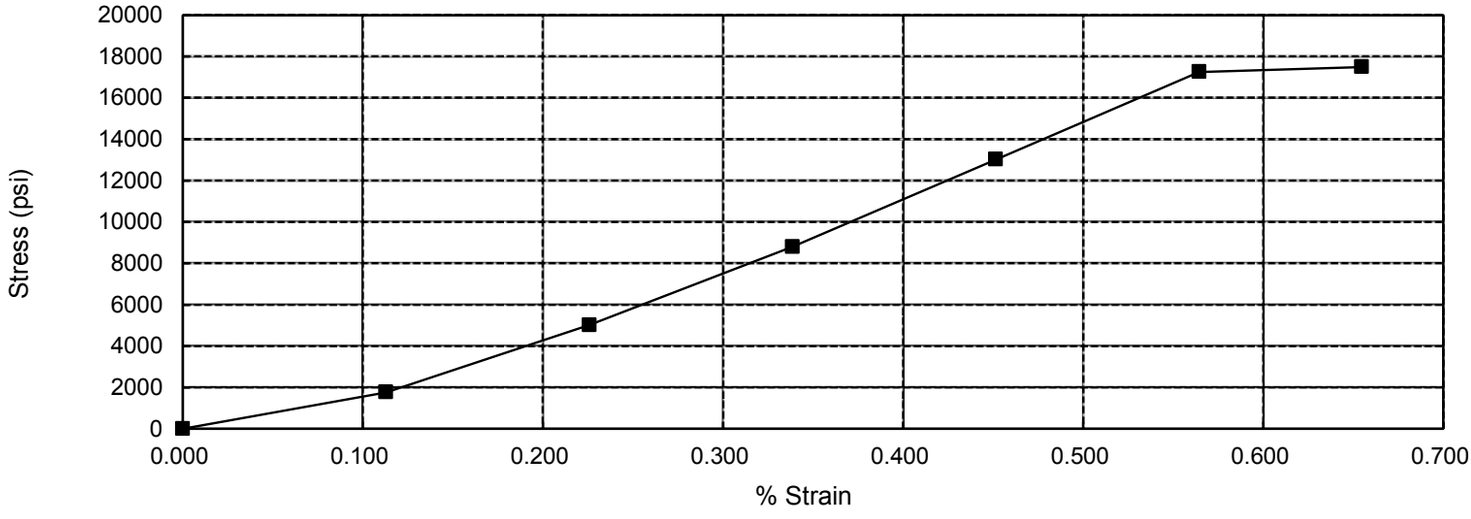
July 7, 2017

Project Name: Ashe Bridge 122
Project Number: G16021.04
Sample ID.: RS-1
Location: B-01
Depth (ft): 5.4-5.7

Length (in.): 4.43
Diameter (in.): 1.87
Area (in²): 2.746
L/D 2.37
Unit Weight (pcf): 181.0

Compressive Strength (psi): 17490
Time to Failure, mins:sec: 6:15

Deflection (in.)	Strain (%)	Load (lbf)	Compressive Strength (psi)	Young's Modulus (psi)
0.000	0.000	0	0	
0.005	0.113	4870	1770	1,568,220
0.010	0.226	13800	5020	2,223,860
0.015	0.339	24180	8800	2,598,933
0.020	0.451	35750	13020	2,883,930
0.025	0.564	47390	17250	3,056,700
0.029	0.655	48040	17490	2,671,748



*Young's modulus is calculated using the secant modulus at each data point per Figure 2 (C) in ASTM D 7012

John Dailly NCDOT CERT No. 105-03-0803

FALCON ENGINEERING

UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE SPECIMENS

Performed in General Accordance with ASTM D7012

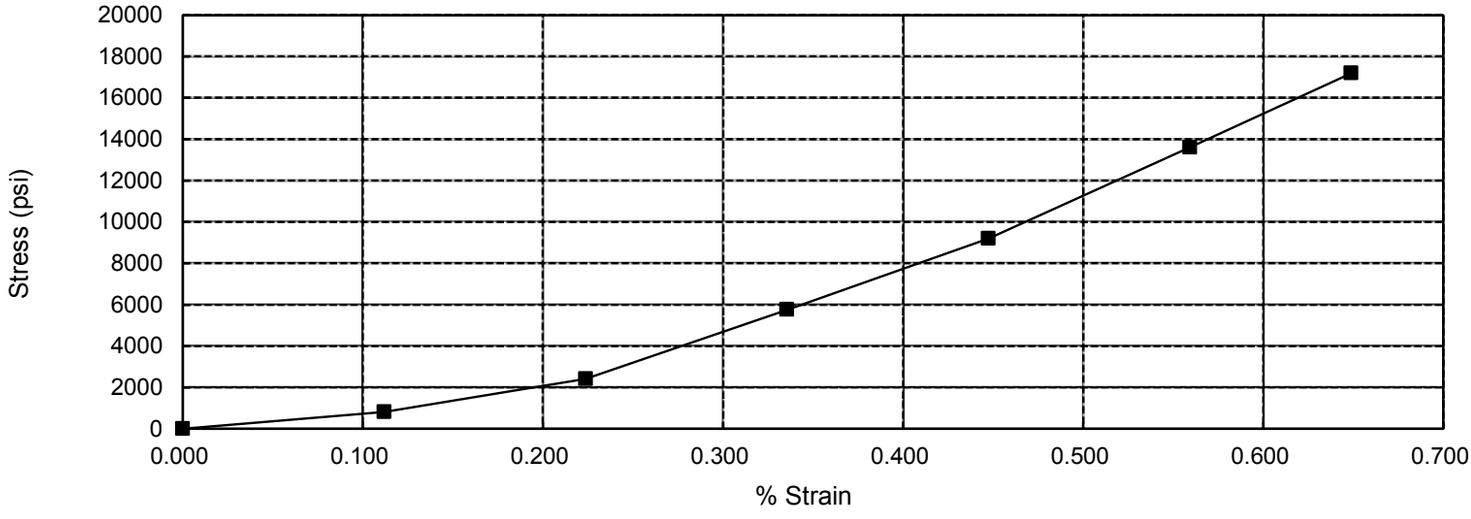
July 7, 2017

Project Name: Ashe Bridge 122
Project Number: G16021.04
Sample ID.: RS-2
Location: B-02
Depth (ft): 7.3-7.8

Length (in.): 4.47
Diameter (in.): 1.87
Area (in²): 2.746
L/D 2.39
Unit Weight (pcf): 187.0

Compressive Strength (psi): 17190
Time to Failure, mins:sec: 6:10

Deflection (in.)	Strain (%)	Load (lbf)	Compressive Strength (psi)	Young's Modulus (psi)
0.000	0.000	0	0	
0.005	0.112	2260	820	733,080
0.010	0.224	6660	2420	1,081,740
0.015	0.336	15830	5760	1,716,480
0.020	0.447	25260	9200	2,056,200
0.025	0.559	37370	13610	2,433,468
0.029	0.649	47200	17190	2,649,631



*Young's modulus is calculated using the secant modulus at each data point per Figure 2 (C) in ASTM D 7012

John Dailly NCDOT CERT No. 105-03-0803